


VIRGINIA DEPARTMENT OF TRANSPORTATION

MATERIALS DIVISION

MEMORANDUM

GENERAL SUBJECT: Revision to Virginia Test Method 140	NUMBER: MD 462-23
SPECIFIC SUBJECT: Adding an example calculation to Section 9 of VTM 140 (Determination of Unconfined Compressive Strength of Cohesive Soil to Estimate Resilient Modulus)	DATE: July 28, 2023
	SUPERSEDES:
APPROVED: Babish Charles dax24016	Charles A. Babish, PE State Materials Engineer  Digitally signed by Babish Charles dax24016 Date: 2023.07.28 14:11:46 -04'00'

EFFECTIVE DATE

- This memorandum is effective August 1, 2023.

PURPOSE/NEED/SCOPE/REQUIREMENTS

- To add an example calculation to VTM 140 to provide clarity on the input parameters used in the correlations shown in Section 9 below.

Changes are **BLUE**

PROCEDURES

9. Report

- 9.1.4. Unconfined compressive strength and shear strength in psi (to nearest 0.1 psi)
- 9.1.8. Liquid limit, plastic limit, and plasticity index in accordance with VTM-7
- 9.1.13. Particle-size analysis in accordance with T 88
- 9.1.15. Maximum dry density and optimum water content in accordance with VTM-1
- 9.1.16. Report the specimen preparation method used: static or impact
- 9.1.17. Resilient modulus in psi (to nearest 1 psi) calculated using the following correlation:

Sample preparation method	Correlation
Static compaction	$M_r = 7884.2 + 99.7 \times Q_u + 193.1 \times PI - 47.9 \times P_{200}$
Impact compaction	$M_r = 6113 + 95.1 \times Q_u + 173.7 \times PI - 27.8 \times P_{200}$

Where M_r = Resilient Modulus (psi) at confining stress of 2 psi and deviator stress of 6 psi; Q_u = Unconfined Compressive Strength (psi); PI = Plasticity Index (%); and P_{200} = % passing No. 200 sieve

Example Calculation:

(a) For Static Compaction Sample:

Given:

$Q_u = 42.4$ psi

$PI = 15$ (Note: For non-plastic soils, $PI = 0$)

$P_{200} = 51.3\%$

So,

$$\begin{aligned}
 M_r &= 7884.2 + 99.7 \times Q_u + 193.1 \times PI - 47.9 \times P_{200} \\
 &= 7884.2 + 99.7 \times 42.4 + 193.1 \times 15 - 47.9 \times 51.3 \\
 &= 12,551 \text{ psi}
 \end{aligned}$$

(b) For Impact Compaction Sample:

Given:

$Q_u = 53.7$ psi

$PI = 15$ (Note: For non-plastic soils, $PI = 0$)

$P_{200} = 51.3\%$

So,

$$\begin{aligned}
 M_r &= 6113 + 95.1 \times Q_u + 173.7 \times PI - 27.8 \times P_{200} \\
 &= 6113 + 95.1 \times 53.7 + 173.7 \times 15 - 27.8 \times 51.3 \\
 &= 12,399 \text{ psi}
 \end{aligned}$$

NOTES

REFERENCES

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